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# Soil classification aashto m145 pdf

Standard Drawing and Specification Programs  
January 2000

## Classification of Soil and Soil-Aggregate Mixtures For Highway Construction Purposes

AASHTO M-145-91 (2000)

**Key Elements:**

- Determine sieve analysis. Determine sieve analyses using AASHTO T-11 and AASHTO T-27 test procedures (Note 1). The 2-mm (No. 10) sieve, 425- $\mu$ m (No. 40) sieve, and 75- $\mu$ m (No. 200) sieve must be included to determine the particle size distribution as a basis for classification.
- Determine the liquid limit. Determine the liquid limit of the material using AASHTO T-39 test procedure.

- Determine the plastic limit. Determine the plastic limit and plasticity index of the material using AASHTO T-70 test procedure.

- Determine classification of material. Using the test limits shown in Table 1 or AASHTO M-145, make the classification of the material. If the material of interest is not included in a further subdivision of the group indicated in Table 1 or Table 2 of AASHTO M-145 (A-3), then proceed test data available, proceed from left to right in Table 1 or Table 2 and the correct group will be found by process of elimination (A-2). The first group from the left into which the test data will fit is the correct classification (A-2).

- Report classification. All limiting test values are shown as whole numbers. If fractional numbers appear on test reports, convert to the nearest whole number for purposes of classification (A-2).

### DESCRIPTION OF SOIL CLASSIFICATION GROUPS:

**Soil fractions:** According to the AASHTO system soils are divided into two major groups as shown in Table 1 of AASHTO M-145. These are the granular materials with 15 percent or less passing the 75- $\mu$ m (No. 200) sieve (A-1) and the silt-clay materials with more than 15 percent passing the 75- $\mu$ m (No. 200) sieve (A-2). Moreover, five soil fractions are recognized and often used in word descriptions of a material. These five fractions are defined as follows:

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Table 4.1 AASHTO Soil Classification System

General classification	Granular materials (35% or less passing US No. 200 sieve)		Silt-clay materials (More than 35% passing US No. 200 sieve)								
			A-1			A-2		A-4	A-5	A-6	A-7
Group classification	A-1a	A-1b		A-2-4	A-2-5	A-2-6	A-2-7			A-7-5	A-7-6
Sieve analysis											
Percent passing											
US No. 10 (2 mm)	50 max										
US No. 40 (420 $\mu$ )	30 max	50 max	51 max	35 max	35 max	35 max	35 max	36 min	36 min	36 min	36 min
US No. 200 (75 $\mu$ )	15 max	25 max	10 max	35 max	35 max	35 max	35 max	36 min	36 min	36 min	36 min
Characteristics of fraction passing US No. 40 (420 $\mu$ )											
Liquid limit											
Plasticity index											
	6 max		Non-plastic	40 max	41 min	40 max	41 min	40 max	41 min	40 max	41 min
				10 max	10 max	11 min	11 min	10 max	10 max	11 min	11 min
Group index	0	0	0	4 max				8 max	12 max	16 max	20 max
Usual types of significant constituent materials	Stone fragments gravel and sand	Fine Sand	Silty or clayey gravel and sand					Silty soils			Clayey soils
General rating as subgrade	Excellent to good				Fair to poor						

Note: A-8 is identified by visual classification, and is not shown in the Table.

Classification procedure: Proceeding from left to right in the chart, the correct group will be found by the process of elimination. The first group from the left consistent with the test data is the correct classification. A-7 group is subdivided into A-7-5 or A-7-6 depending on the plastic limit. For  $w_p < 30$ , the classification is A-7-6; for  $w_p \geq 30$ , it is A-7-5.

Table 2-1. Grading of ILS fine and coarse blends and Grading E and A of AASHTO M-145

Sieve Size	Fine w/Clay	Fine w/Silt	Grading E	Coarse w/Clay	Coarse w/Silt	Grading A
1"	100.0	100.0	100	100.0	100.0	100
1/2"	100.0	100.0	100	90.0	90.0	-
3/8"	100.0	100.0	100	64.0	64.0	36-65
# 4	99.3	99.3	55-80	45.9	45.9	25-55
# 8	45.2	46.2	-	29.8	30.8	-
# 10	41.6	42.5	40-60	23.6	24.6	15-40
# 16	22.5	23.1	25-50	11.3	11.3	5-20
# 200	7.1	6.9	6-20	7.0	7.0	2-8

Table 2-2. Sources and classifications of ILS soil-aggregate blends according to AASHTO M-145

Soil-Aggregate Type	Soil-Aggregate Classification (AASHTO M-145)	Materials	Source	Classification	
				A-3	A-1
Fine-Graded (Grading E of AASHTO M-145)	Crushed Limestone (particle size less than #4 and retained on #8)	Lafarge Frederick, MD			
	Wastewater Sand (Natural Sand Passing #8)	Aggtrans in Hanover, MD			
	Lean Clay (CL)	Aggregate Transport Corporation in Hanwood, MD			
Coarse-Graded (Grading A of AASHTO M-145)	Silt (M.)	U.S. Army Corps of Engineers, Wataways Experimental Station in Vicksburg, MS			
	Crushed Limestone	Lafarge Frederick, MD			
	Manufactured Fine Aggregate (Limestone Dust)	Lafarge Frederick, MD			
A1	Lean Clay (CL)	Aggregate Transport Corporation in Hanwood, MD			
	Silt (M.)	U.S. Army Corps of Engineers, Wataways Experimental Station in Vicksburg, MS			

## 2.2 Preliminary Study of AASHTO T180

A preliminary study was conducted at the AMRL laboratory to examine the compressibility of ILS and the variability of the measured density and optimum moisture contents. Three replicates of each of the four materials were compacted using a 4.54 kg manually-operated rammer according to procedures B and D of AASHTO T 180. Prior to the compaction, specific gravity of the soil-aggregate blends were determined according to AASHTO T 84 and T 85 [1]. The specific gravity values were used to calculate the percentage of water at 100% saturation of the blends. The measured specific gravities are provided in Table 2-3.

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٢٣	٢٩	٢٩	٢٣	٣ ..	٣١٣
٦٦	٦٦	٦٦	٦٦	٣ ..	٣١٣
					النسبة المئوية (%)
					٣١٣

Figure 2-1: Results of the preliminary study of AASHTO T180

Table 2-3: Specific Gravity of ILS Soil-Aggregate Blends

Table 2-4: Compaction Test Results of ILS Soil-Aggregate Blends

Table 2-5: Optimum Moisture Content of ILS Soil-Aggregate Blends

Table 2-6: Dry Density of ILS Soil-Aggregate Blends

Table 2-7: Compressive Strength of ILS Soil-Aggregate Blends

Table 2-8: Specific Gravity of ILS Soil-Aggregate Blends

Table 2-9: Optimum Moisture Content of ILS Soil-Aggregate Blends

Table 2-10: Dry Density of ILS Soil-Aggregate Blends

Table 2-11: Compressive Strength of ILS Soil-Aggregate Blends

Table 2-12: Specific Gravity of ILS Soil-Aggregate Blends

Table 2-13: Optimum Moisture Content of ILS Soil-Aggregate Blends

Table 2-14: Dry Density of ILS Soil-Aggregate Blends

Table 2-15: Compressive Strength of ILS Soil-Aggregate Blends

Table 2-16: Specific Gravity of ILS Soil-Aggregate Blends

Table 2-17: Optimum Moisture Content of ILS Soil-Aggregate Blends

Table 2-18: Dry Density of ILS Soil-Aggregate Blends

Table 2-19: Compressive Strength of ILS Soil-Aggregate Blends

Table 2-20: Specific Gravity of ILS Soil-Aggregate Blends

Table 2-21: Optimum Moisture Content of ILS Soil-Aggregate Blends

Table 2-22: Dry Density of ILS Soil-Aggregate Blends

Table 2-23: Compressive Strength of ILS Soil-Aggregate Blends

Table 2-24: Specific Gravity of ILS Soil-Aggregate Blends

Table 2-25: Optimum Moisture Content of ILS Soil-Aggregate Blends

Table 2-26: Dry Density of ILS Soil-Aggregate Blends

Table 2-27: Compressive Strength of ILS Soil-Aggregate Blends

Table 2-28: Specific Gravity of ILS Soil-Aggregate Blends

Table 2-29: Optimum Moisture Content of ILS Soil-Aggregate Blends

Table 2-30: Dry Density of ILS Soil-Aggregate Blends

Table 2-31: Compressive Strength of ILS Soil-Aggregate Blends

Table 2

Trial Pit no.	Station	Soil Class (AASHTO)	Dry Density (lb/ft <sup>3</sup> )	Moisture Content (%)
28	32+700	A-4	109.75	4.0
30	33+350	A-1-a	106.25	3.4
31	33+730	A-1-b	107.56	3.5
33	34+220	A-1-a	126.60	6.5
35	34+920	A-4	110.81	3.8
37	35+760	A-2-4	114.43	2.6
38	36+080	A-1-a	109.75	3.7
39	36+620	A-1-a	118.11	5.2
41	37+130	A-1-a	116.24	3.3
43	38+102	A-1-b	113.87	3.6
43-A-C	38+250	A-1-b	124.23	2.5
45-A-C	39+200	A-1-a	128.66	4.0
46-A-C	39+560	A-1-b	120.61	4.1
78	58+640	A-1-b	98.64	8.2
79	58+820	A-1-a	127.73	4.2
85	60+680	A-1-a	129.98	5.3
86	61+080	A-1-b	129.41	4.5
88	61+880	A-1-b	131.29	4.6

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